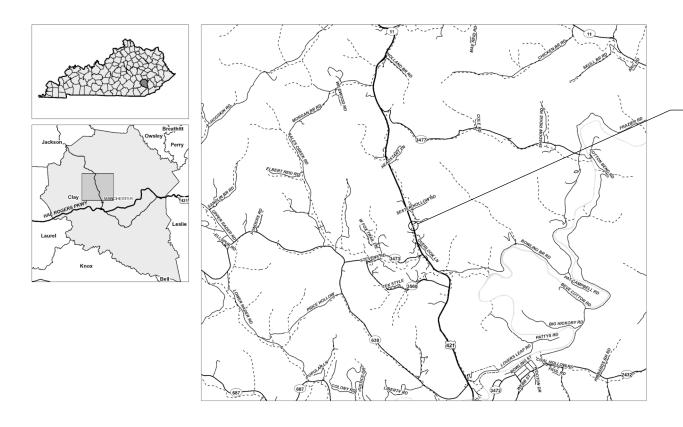
KENTUCKY TRANSPORTATION CABINET DEPARTMENT OF HIGHWAYS BRIDGE REHABILITATION PLANS



LOCATION MAP

026B00002N US-421 OVER BRANCH OF ISLAND CREEK 37.19277777, -83.779722222

	INDEX OF SHEETS
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S1	TITLE & LOCATION MAP
S2	GENERAL NOTES
S3	LAYOUT
S4	TYPICAL SECTIONS
S5	MAINTENANCE OF TRAFFIC
S6	SOUTH ABUTMENT
S7	NORTH ABUTMENT
S8	ABUTMENT DETAILS
S9	ABUTMENT DETAILS
S10	DECK AND FRAMING PLAN
S11	PPC I-BEAM, TYPE I DETAILS
S12	DIAPHRAGM DETAILS
S13	CONSTRUCTION ELEVATIONS
S14	MODIFIED RAILING SYSTEM TYPE T631
	SPECIAL NOTES

Bridge Overlay Approach Pavement Concrete Sealing

Erosion Prevention and Sediment Control

Structures With Over The Side Drainage Traffic Control on Bridge Repair Contracts

Utilities and Rail Certification

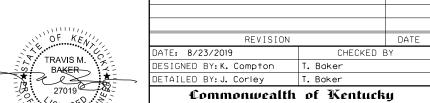
STANDARD DRAWINGS

RBI-001-12	Typical Guardrail Installations
RBR-001-13	Steel Beam Guardrail ("W" Beam)
RBR-015-06	Steel Guardrail Posts
RBR-055-01	Delineators for Guardrail
RBR-018	Guardrail System Transition
BHS-011	Railing System Side Mounted MGS Details
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BBP-001-12	Elastomeric Bearing Pads for Prestressed Beams
BBP-002-04	Bearing Details
BJE-001-13	Neoprene Expansion Dams and Armored Edges
RBM-115-10	Concrete Barrier Wall Type 9T (Temporary)
RBR-005-11	Guardrail Components
RBR-010-06	Guardrail Terminal Sections
RBR-016-06	Timber Guardrail Posts
RGX-050-02	Gabion Retaining Walls
•	
•	

SPECIFICATIONS

2019 Standard Specifications for Road and Bridge Construction.

AASHTO LRFD Bridge Construction Specifications with Current Interims.



DEPARTMENT OF HIGHWAYS

CLAY

BRANCH OF ISLAND CREEK

TITLE & LOCATION MAP

BRIDGE NUMBER

026B00002N

AECOM



<u>DESIGN LOAD</u>: This superstructure is designed for KY-HL93 Live Load, (i.e. 1.25xAASHTO HL93 live load). This bridge is designed for a future wearing surface of 15 psf.

<u>DESIGN METHOD</u>: All superstructure members are designed to be equivalent or greater than the load and resistance factor design method as specified in the current AASHTO Specifications.

ON-SITE INSPECTION: Each Contractor submitting a bid for this work shall make a thorough inspection of the project site prior to submitting a bid and shall be thoroughly familiarized with existing conditions so that work can expeditiously performed after a Contract is awarded. Submission of a bid will be considered evidence of this inspection having been made. All claims resulting from the site conditions will not be honored by the Department of Highways.

<u>VERIFYING FIELD CONDITIONS</u>: Dimensions shown on these Plans are taken from field measurements. The Plan dimensions and details relative to the existing structure are subject to nominal construction variations. It shall be the Contractor's responsibility to verify such dimensions and details in the field and make the necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in the scope of the work; however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work. In addition, the overrun and underrun formulas may be applied to appropriate repairs provided that the requirement of Article 104.02.02 of the Standard Specifications is satisfied.

<u>PLANS OF EXISTING STRUCTURE</u>: Plans of the existing structure are not available. Existing superstructure is assumed to be constructed according to Standard Drawing G-24. The completeness of these drawings is not guaranteed and no responsibility is assumed for their accuracy.

<u>CONSTRUCTION LOAD</u>: The Contractor shall abide by the posted bridge limits. Storage of material on the bridge is prohibited.

CONSTRUCTION IDENTIFICATION: The names of the Prime Contractor and the Sub-Contractor shall be imprinted in the concrete with l'letters at a location designated by the Engineer. The Contractor shall furnish all plans, equipment, and labor necessary to do the work for which no direct payment will be

<u>UTILITIES</u>: Before beginning work, locate all existing utilities. Consider location of utilities shown on the drawings to be approximate and for informational purposes only. The Department does not warrant the locations and assumes no responsibility for the accuracy or completeness. The Contractor must make his own determination. Except as shown on the Plans, work around and do not disturb existing utilities.

<u>DAMAGE OUTSIDE CONSTRUCTION LIMITS</u>: Any area used outside the environmentally cleared area shall obtain full environmental approvals prior to use. Once cleared, any area that is disturbed outside of the limits of the construction during the life of the project shall be repaired by the Contractor at his expense, should any damage result from the Contractor's actions.

DAMAGE TO THE STRUCTURE: The Contractor shall bear full responsibility and expense for repair of any and all damage to the structure, should such damage result from the Contractor's actions. The Contractor is completely responsible for the stability of the structure from the time of mobilization until after the bridge has been reopened to normal traffic following completion of all work required in the Contract. After completion of all operations, the structure and site shall be left in a condition that is in accordance with Section 105.12 of the Specifications.

<u>DIMENSIONS:</u> Dimensions are for a normal temperature of 60 degrees fahrenheit. Layout dimensions are horizontal dimensions.

GENERAL NOTES

REMOVE SUPERSTRUCTURE: This pay item for "Remove Superstructure" shall consist of the removal of the superstructure (beams and deck/asphalt), and partial removal of the abutments and wingwalls as shown in the Plans. Portions of the existing abutments and wingwalls shall remain in place to be reused in the rehabilitated structure. Care shall be exercised not to damage areas of remaining concrete or reinforcing steel during concrete removal operations.

Remove concrete by means of approved pneumatic hammers employing pointed and blunt chisel tools. Hydraulic hoe-ram type hammers will not be permitted. The weight of the hammer shall not be more than 35 pounds for removal within 18 inches of portions to be preserved. Outside the 18 inch limit, the Contractor may use hammers not exceeding 90 pounds upon the approval of the Engineer. Do not place pneumatic hammers in direct contact with reinforcing steel that is to be retained. Care shall be taken to not damage bond to adjacent non-exposed reinforcing steel during concrete removal processes. The perimeter of all areas where concrete is removed shall be tapered at an approximately 45° angle, except that the outer edges of all chipped areas shall be saw cut to minimum depth of linch to prevent feather edging unless otherwise approved by the Engineer. After all concrete has been removed, the repair surface shall be prepared by abrasive blast cleaning. Abrasive blast cleaning shall remove all fractured surface concrete and all traces of any unsound material or contaminants such as oil, arease, dirt, slurry, or any materials which could interfere with the bond of freshly placed concrete. The Contractor shall dispose all removed material off state right of way in an approved site.

WELDING REINFORCEMENT: The welding and welding material shall conform to the "Recommended Practices for Welding Reinforcing Steel", American Welding Society Specifications, Current Edition. No direct payment shall be made for welding or weld material, but the cost of these items shall be included in the unit price bid for the repair being completed.

<u>DISPOSAL OF MATERIALS</u>: All materials and debris removed from or beneath the bridge shall become the property of the Contractor and shall be removed from the right-of-way.

COMPLETION OF THE STRUCTURE: The Contractor is required to complete the structure in accordance with the Plans and Specifications. Material, labor, or construction operations, not otherwise specified, are to be included in the bid item most appropriate for the work involved and otherwise considered incidental to the Contract. This may include cofferdams, shoring, excavations, backfilling, removal of all or parts of the existing structure, phase construction, incidental materials, labor, or anything else required to complete the structure.

BEFORE YOU DIG: The Contractor shall be responsible for all requirements and conformation with the Underground Facility Damage Prevention Act of 1994. The Contractor will be responsible for locating any utilities on this project. Il underground utilities shall be located prior to construction. Any utilities disturbed or damaged as a result of the Contractor's operations will be repaired to the satisfaction of the utility owner at the Contractor's expense. The Contractor is advised to call (800) 752-6007 a minimum of two working days prior to excavation for information on the location of some, but not necessarily all underground utilities.

B. GENERAL NOTES REHABILITATION PROJECTS MATERIALS FOR DESIGN SPECIFICATIONS:

For Class "A" Concrete: F'C = 3,500 psi For Class "AA" Concrete: F'C = 4,000 psi

For Class "M" Concrete: F'C = 4,000 psi

For Steel Reinforcement: FY = 60,000 psi

The Specifications, Current Edition, as designated below shall govern the following materials furnished:

MaterialSpecificationAnchorBoltsF1554, Grade 55

Grout C1107

Anchor Dowels A311, Grade 1018 Smooth Steel Rods

 $\underline{\text{CONCRETE}};$ Class "AA" Concrete is to be used throughout the superstructure and in the portions of the substructure above the tops of caps. Class "A" concrete is to be used in the substructure below the caps. Prestressed beam concrete shall be in accordance with the plans and specifications.

<u>SUPERSTRUCTURE SLAB</u>: The superstructure slab shall be poured continuously from end to end of slab before the concrete is allowed to set.

<u>REINFORCEMENT</u>: Dimensions shown from the face of concrete to bars are to center of bars unless otherwise shown. Spacing of bars is from center to center of bars. Any reinforcing bars designated by suffix "e' in the plans shall be epoxy coated in accordance with section 811.10 of the Standard Specifications. Any reinforcing bars designated by suffix "s' in a Bill of Reinforcement shall be considered a stirrup for purposes of bend diameters.

<u>EXISTING STEEL REINFORCEMENT</u>: The cost of cutting, bending, and cleaning existing steel reinforcement shall be incidental to the repair item being completed.

<u>BEVELED EDGES</u>: Bevel all exposed edges $\frac{3}{4}$ " unless otherwise noted.

STAY-IN-PLACE METAL FORMS: Stay-In-Place Metal Forms may be used on bridge decks under the following additional conditions:

The valleys of forms shall be filled with trimmed styrofoam to eliminate increased dead load from concrete.

The welding shall be performed by a certified welder.

<u>CONCRETE SEALER:</u> Apply concrete sealer in accordance with the Special Note for Concrete Sealing.

PREFORMED CORK EXPANSION JOINT MATERIAL: Preformed Cork Expansion Joint Material shall conform to subsection 807.04.02 (Type II) of the Kentucky Department of Highways Standard Specifications.

PAYMENT FOR PRECAST CONCRETE BEAMS: The basis of payment for the Prestressed Concrete Beams shall be at the contract unit price per linear foot of beam, in accordance with the specifications.

SHOP DRAWINGS: The fabricator shall submit all required shop plans, by email to SHOP_XXXXxxxxN@docs.e-Builder.net, for review. These submissions shall depict the shop plans in .PDF format, as either II*xI7" or 22"x36" sheets. Designers will make review comments on these electronic submissions as needed and, if required, shall return them to the fabricator for corrections and resubmittal. Upon acceptable reconciliation of all comments, files shall be sent to the Bridging Kentucky Shop Plan Coordinator for distribution. Only plans submitted directly to the Shop Plan Coordinator will be distributed. Additionally, only plans electronically stamped "Distributed by The Bridging Kentucky Program Team" are to be used for fabrication. While this process does not require the submission of paper copies, the Engineer of Record reserves the right to require such copies on a case by case basis.

When any changes to the design plans are proposed, the shop drawings reflecting these changes shall be submitted through the process above.

Note: The designation in the email XXXXxxxxxXN refers to the Bridge ID number which is located on the Title Sheet, Slof the Bridge Plans. Example: SHOP_042B000191N@docs.e-Builder.net

C. JOINT WATERPROOFING AT ABUTMENTS

The joint between the abutment seats and superstructure and between the abutment wings and superstructure shall be waterproofed as detailed on these Plans.

Mastic Tape used to seal joints shall meet the requirements of ASTM C-877 Type I, II, or III. The joint is to be covered with 12-inch wide mastic tape. Prior to application, the joint surface shall be clean and free of dirt, debris, or deleterious material. Primer, if required by the tape manufacturer, shall be applied for a minimum width of 9" on each side of the joint.

Mastic Tape shall be either:

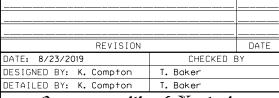
EZ-WRAP RUBBER by PRESS-SEAL GASKET CORPORATION, SEAL WRAP by MAR MAC MANUFACTURING CO. INC.,

CADILLOC by UP RUBBER CO. INC.

or an approved equivalent.

Mastic Tape shall cover the joint continuously unless otherwise shown in the Plans. Mastic Tape shall be spliced by lapping a minimum of 6" and in accordance with the manufacturer's recommendations with the overlap running downhill.

The cost of this work, including all materials, labor, equipment, tools and incidentals necessary for furnishing and installing Mastic Tape shall be considered incidental to the unit price bid for Class AA Concrete and no separate measurement or payment shall be made.



Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS

CLAY

ROUTE US-421

BRANCH OF ISLAND CREEK

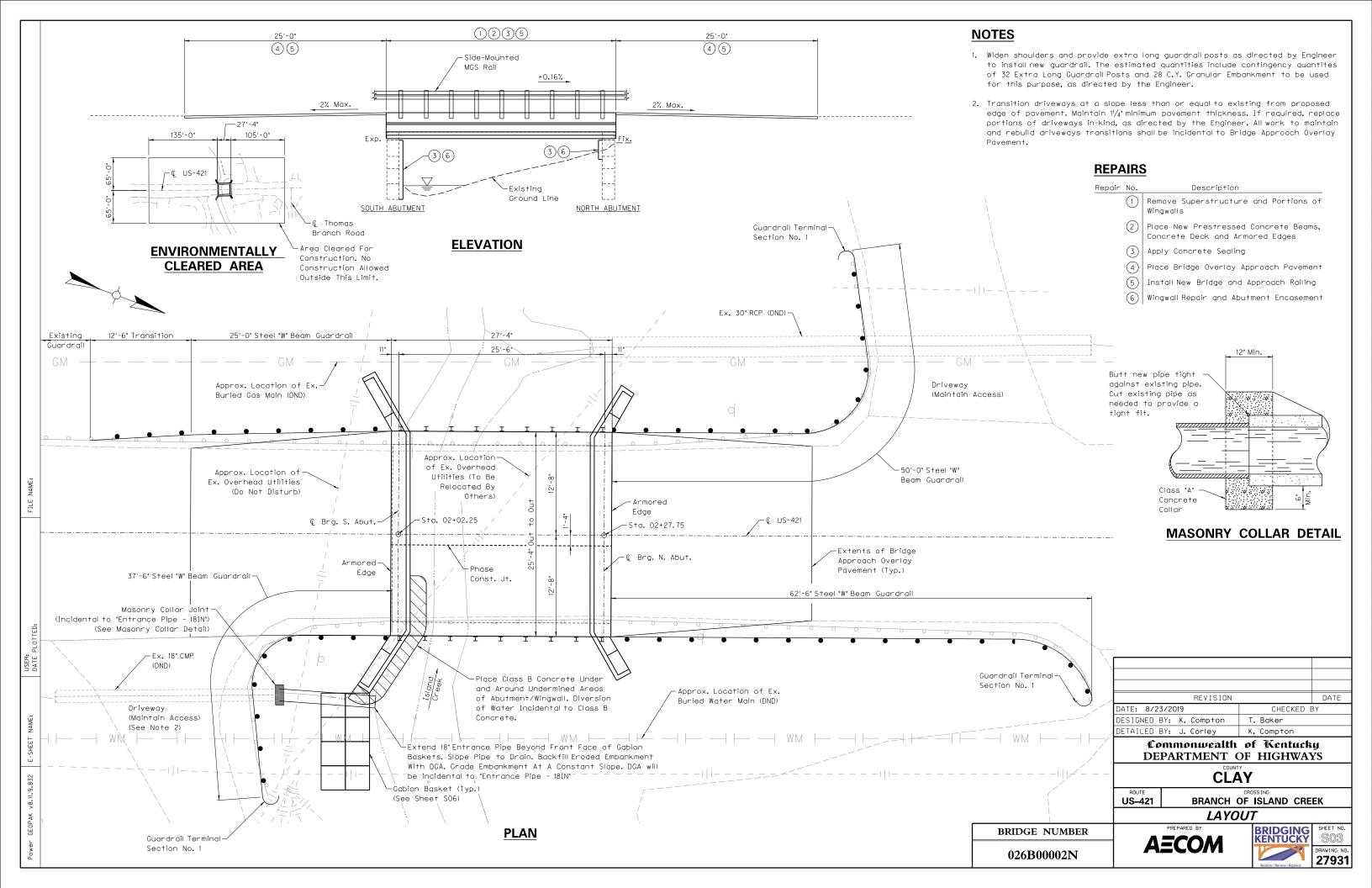
GENERAL NOTES
PREPARED BY

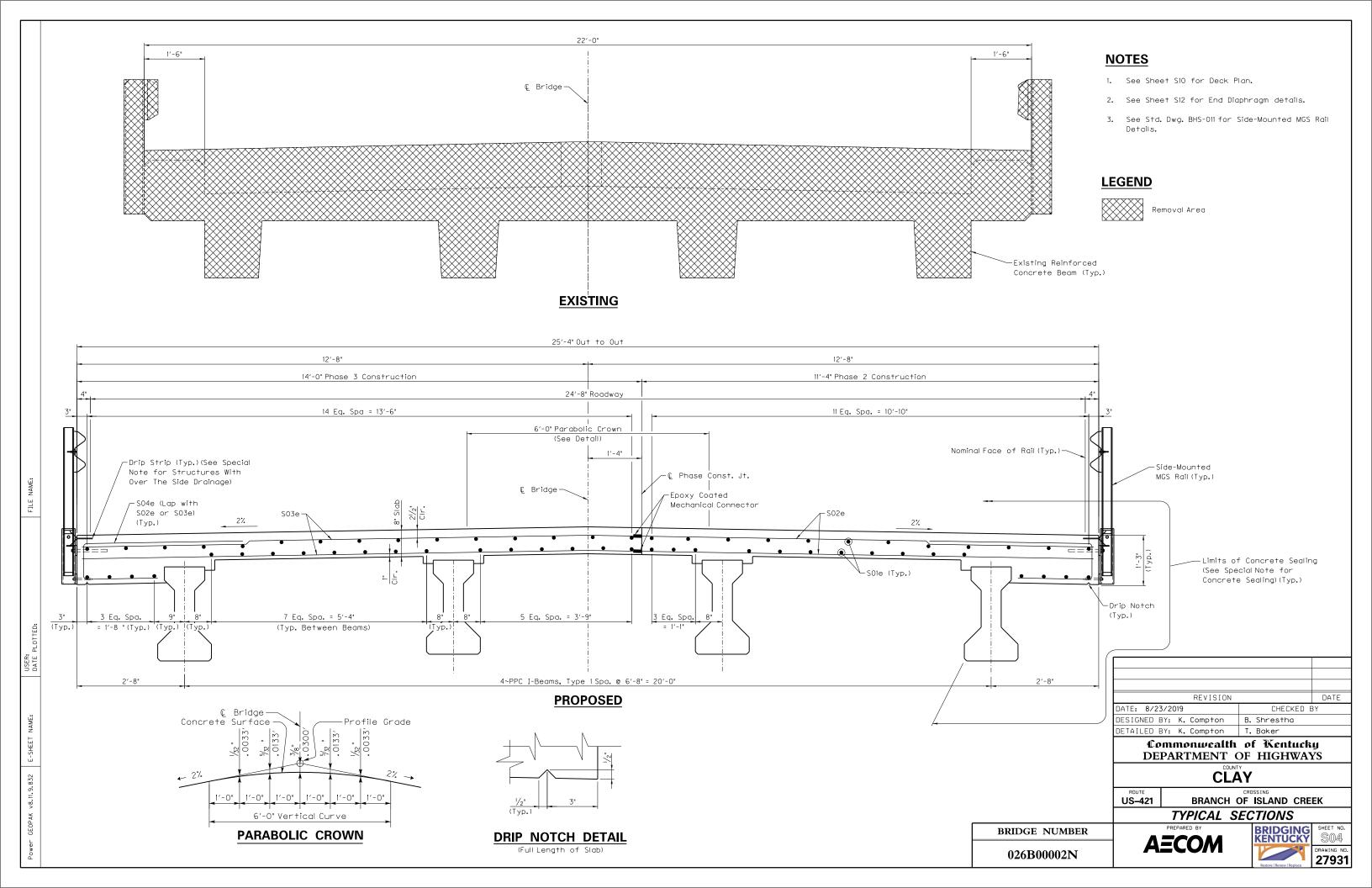


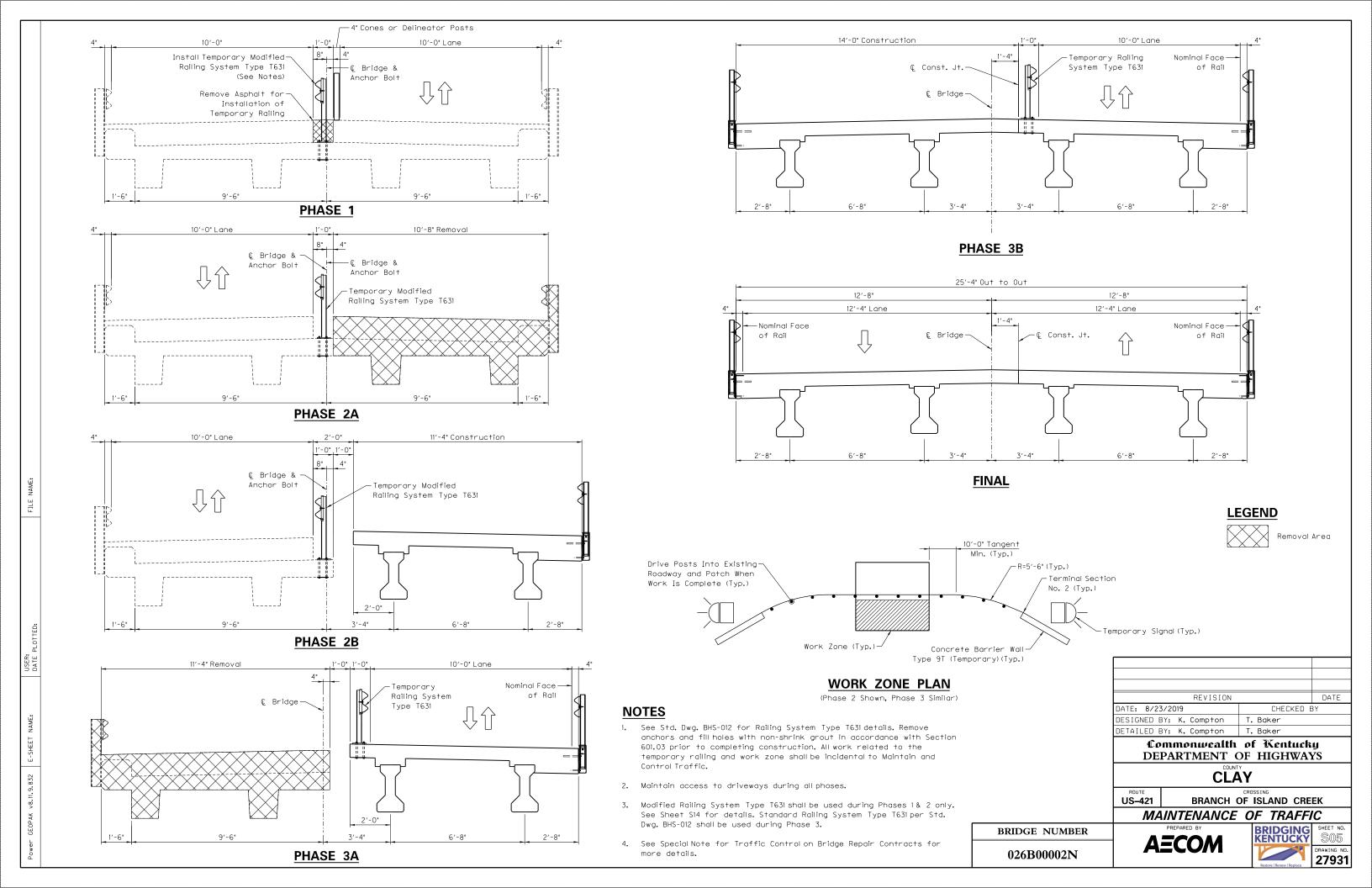


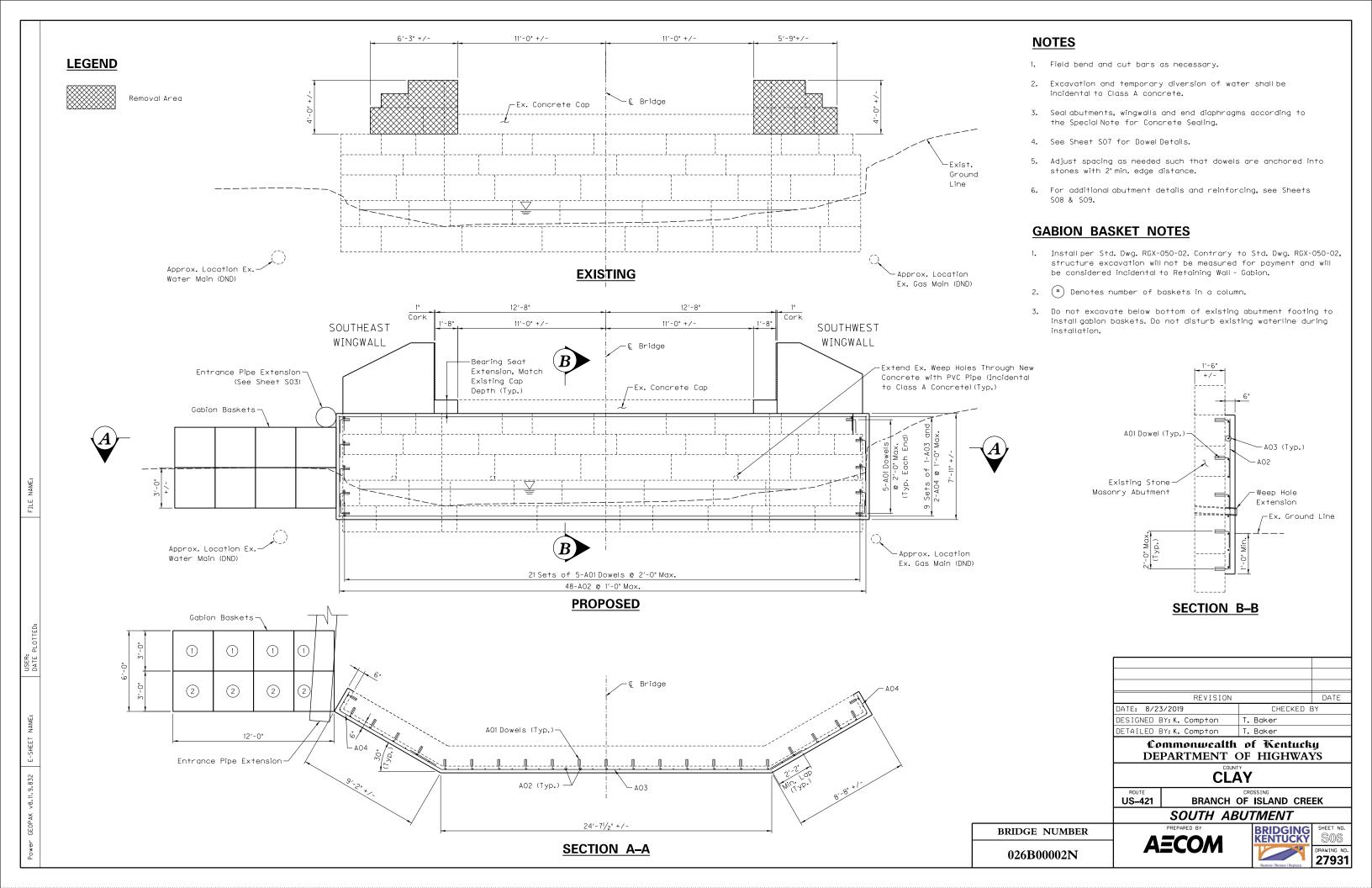
BRIDGE NUMBER

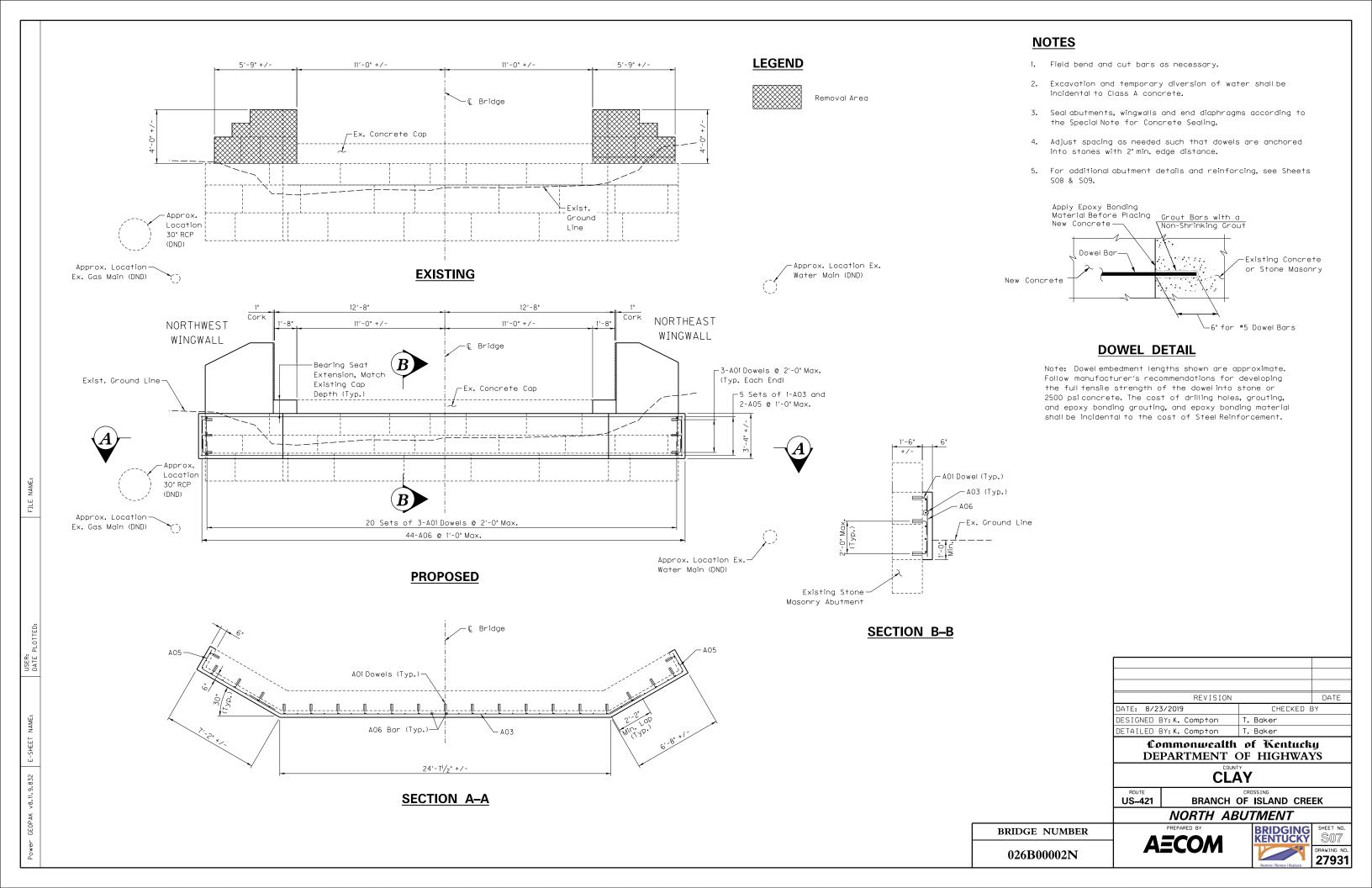
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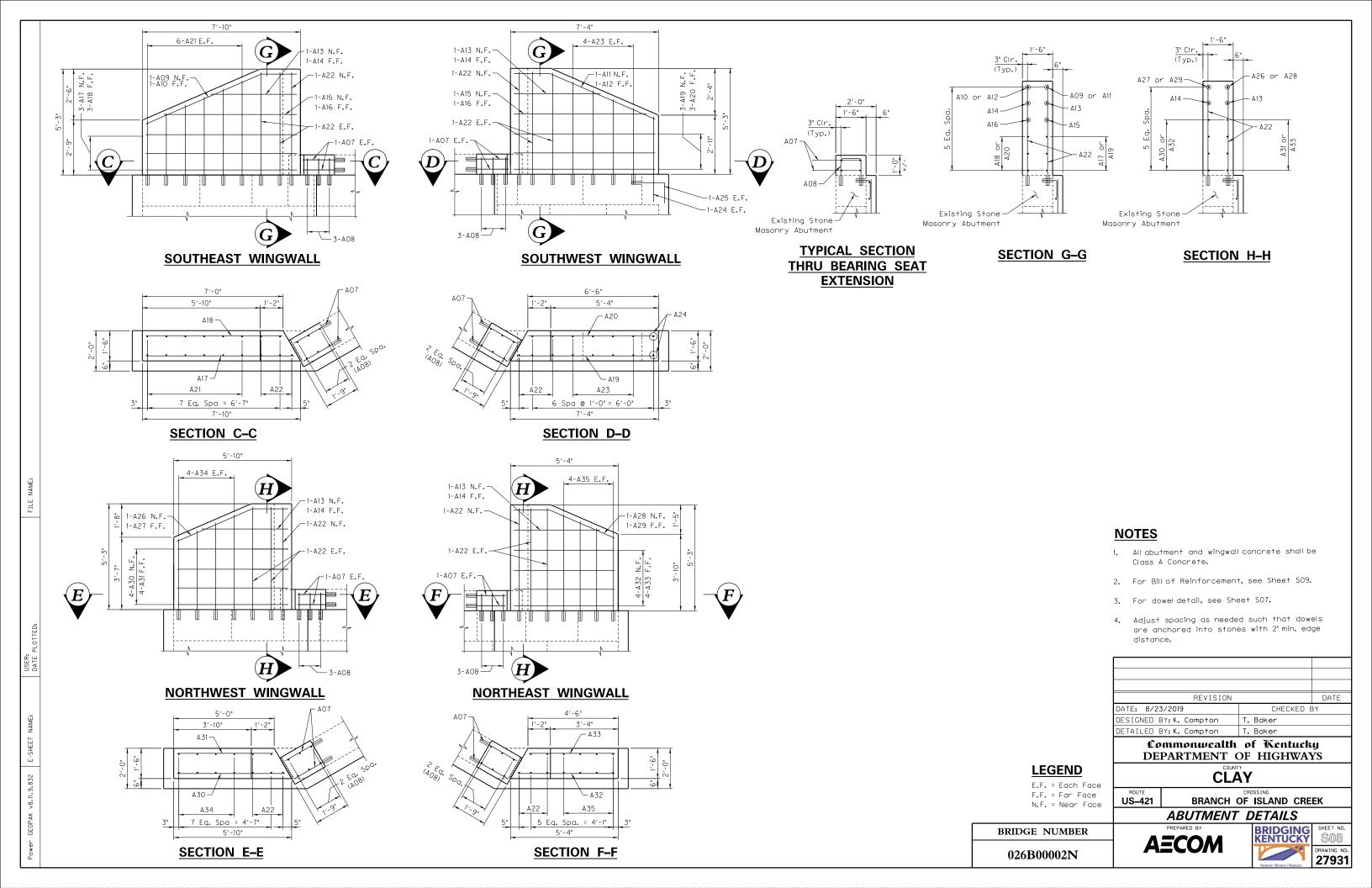


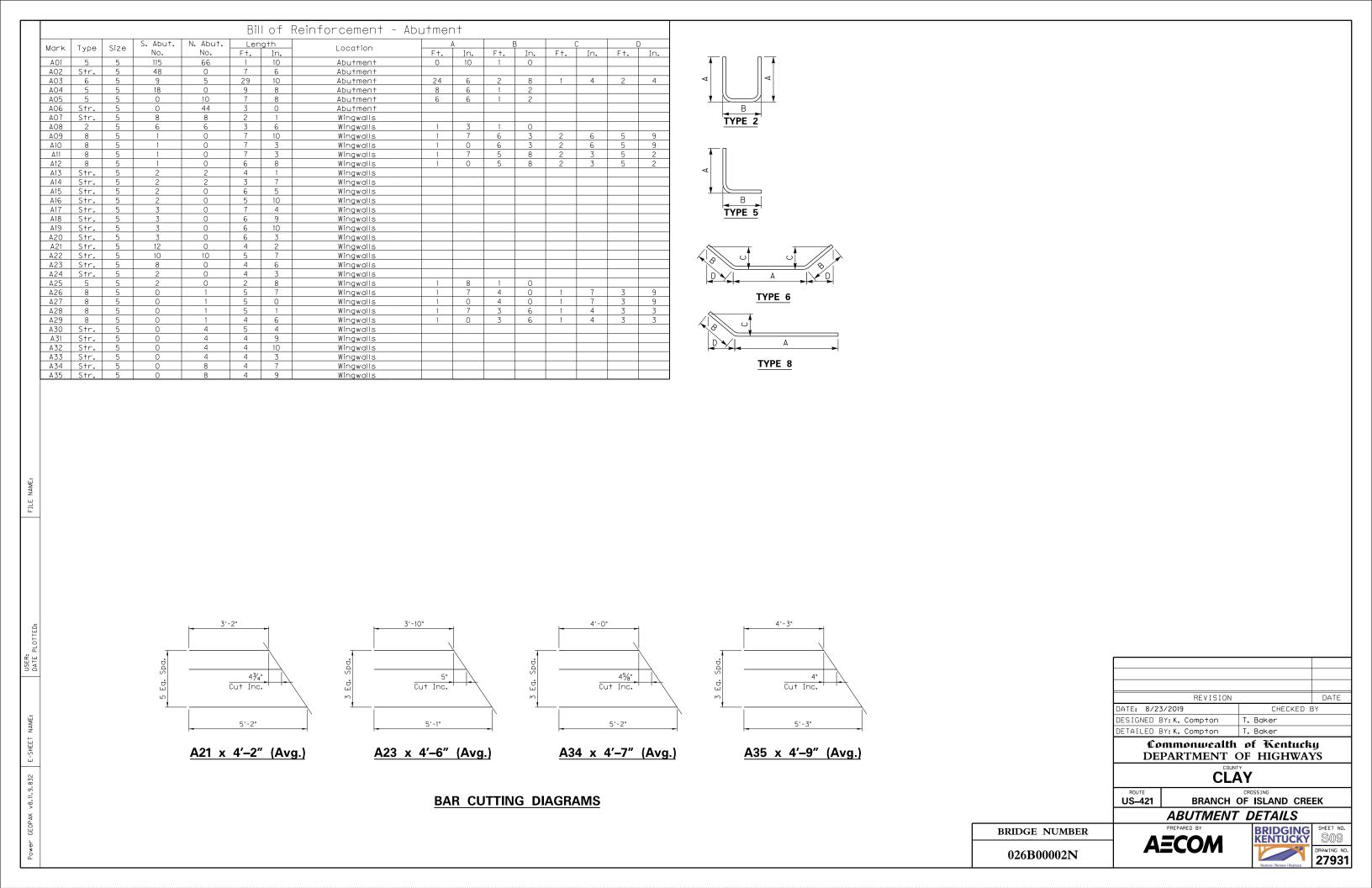


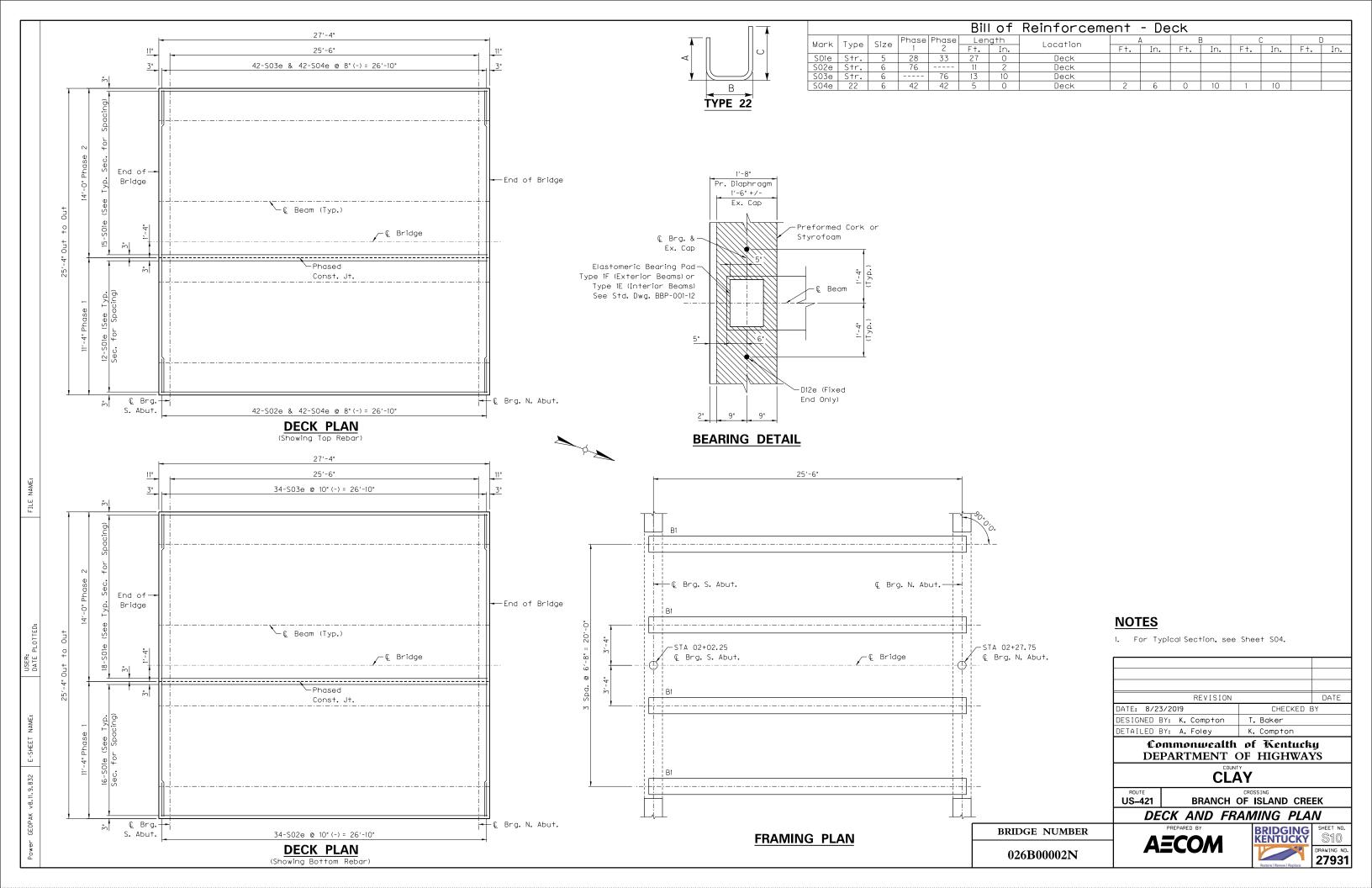


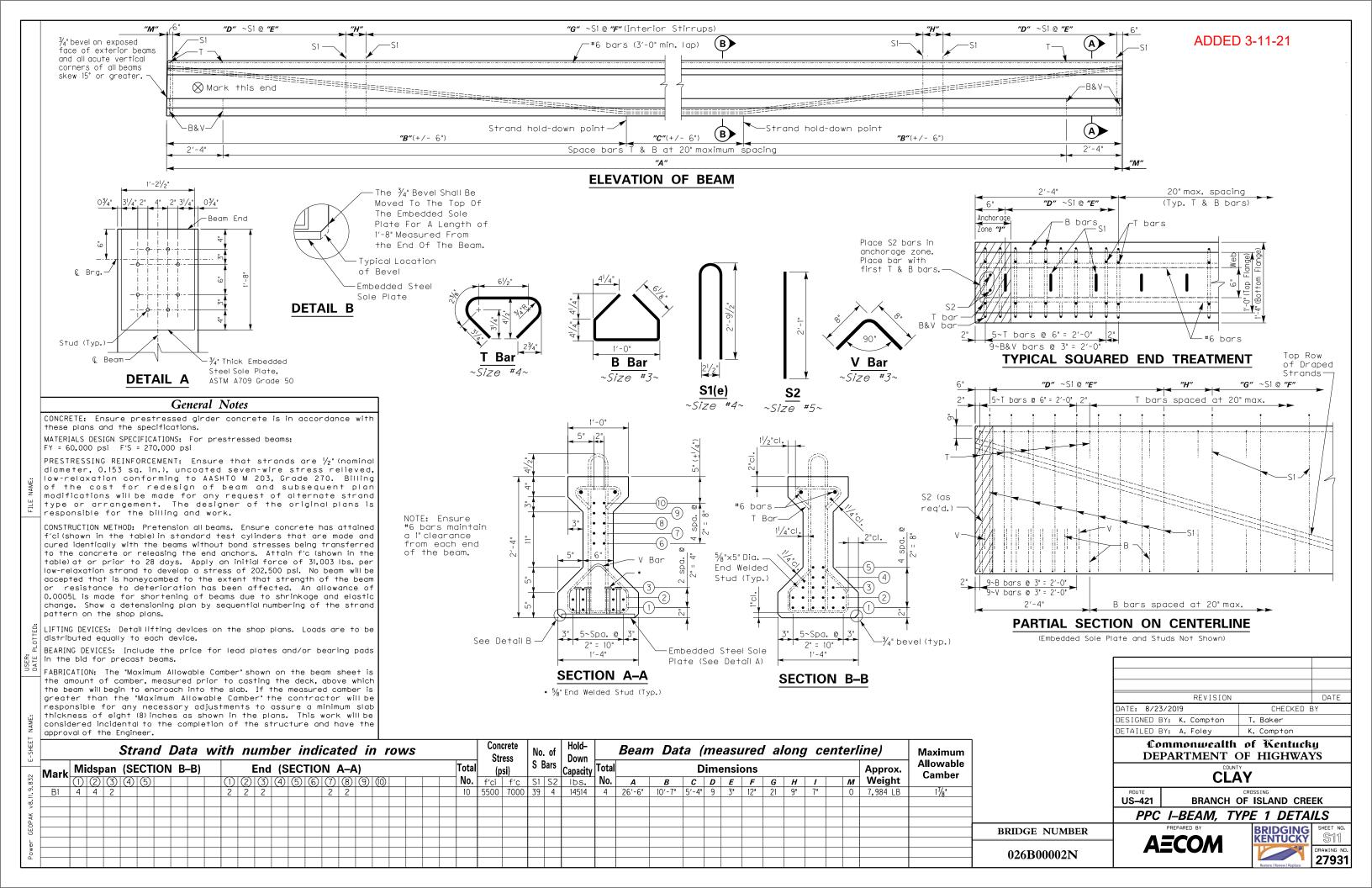


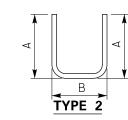






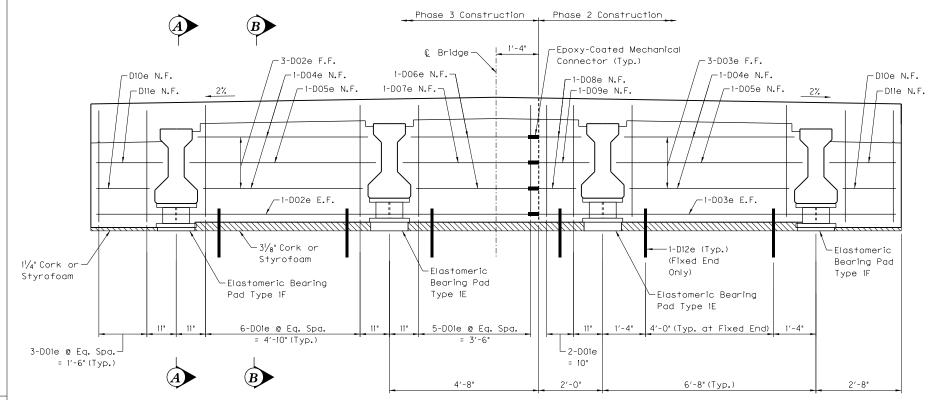


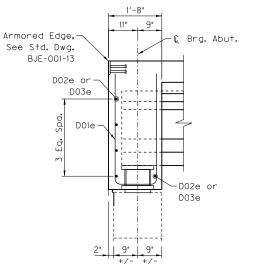


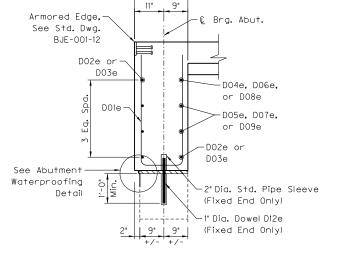


Bill of Reinforcement - Diaphragm															
Mark Type Siz	Size	Phase 2	Phase 3	3 Length Location		A		В		С		[D		
MUIK	Туре	3126	No.	No.	Ft.	In.	Location	F†.	In.	F†.	In.	F†.	In.	F†.	In.
D01e	2	5	22	28	8	0	Diaphragm	3	4	1	4				
D02e	Str.	5		10	13	10	Diaphragm								
D03e	Str.	5	10		11	2	Diaphragm								
D04e	Str.	5	4	4	5	0	Diaphragm								
D05e	Str.	5	2	2	5	10	Diaphragm								
D06e	Str.	5		4	3	10	Diaphragm								
D07e	Str.	5		2	4	3	Diaphragm								
D08e	Str.	5	4		1	2	Diaphragm								
D09e	Str.	5	2		1	7	Diaphragm								
D10e	Str.	5	2	2	1	8	Diaphragm								
Dile	Str.	5	2	2	2	1	Diaphragm								
DI2e	Str.	*	3	3	1	7	Diaphragm								

* D12e - I* Smooth Round Bar, ASTM A311, Grade 1018, Epoxy Coated.







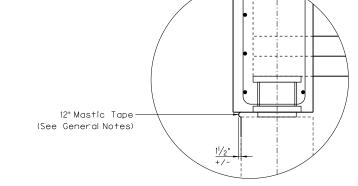
SECTION A-A

SECTION B-B

2" Dia. x 5" Pipe Sleeve shall be closed at one end. Secure pipe sleeve to prevent floating while placing concrete. Sleeve to sit on cork or styrofoam. Pipe sleeve to be incidental to Class "AA" Concrete

ABUTMENT DIAPHRAGM

(North Abutment Diaphragm Shown, South Abutment Similar)



ABUTMENT WATERPROOFING DETAIL

NOTES

- 1. See Std. Dwg. BBP-001-12 for Elastomeric Bearing Pad Details.
- 2. Prior to placing bearings, bridge seat shall be thoroughly cleaned and ground level under bearings.
- 3. Steel load plates and HP posts shall be ASTM A709 Grade 50 Steel and shall be galvanized according to ASTM A123. Load plates shall be vulcanized to the elastomeric pad during the molding process. Cost of Furnishing and installating of bearing pads, load plates, sole plates and HP posts shall be considered incidental to the unit bid price for Precast PC I-Beam, Type 1.
- 4. Control welding so that the plate temperature at the elastomer bonded surface does not exceed 300°F as determined by use of pyrometric sticks or other temperature monitoring devices.

REVISION		DATE			
DATE: 8/23/2019	CHECKED E	3 Y			
DESIGNED BY: K. Compton	T. Baker				
DETAILED BY: K. Compton	T. Baker				
Commonwealth of Kentucky					

DEPARTMENT OF HIGHWAYS

CLAY

US-421 **BRANCH OF ISLAND CREEK**

DIAPHRAGM DETAILS

BRIDGE NUMBER

026B00002N

AECOM



3.18" (Type 1E) 1.20"(Type 1F) Elastomeric

Bearing Pad

BEARING DETAIL

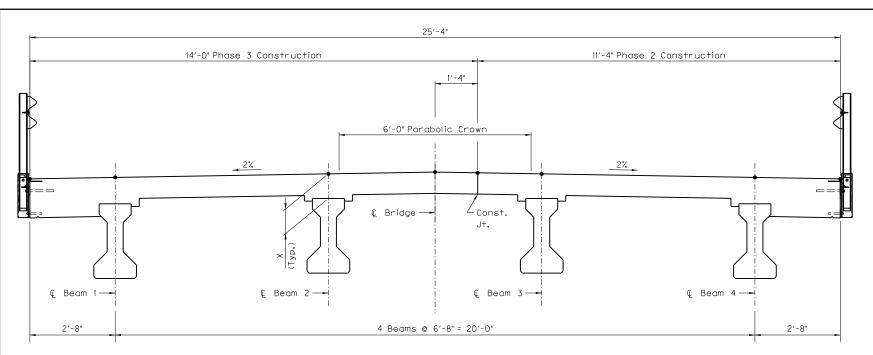
"Dia. Vent Hole in Web of HP10×42

-1'-3"×1'-0"×1¹/₂" Steel Load Plate

 $1'-4"\times1'-0"\times\frac{3}{4}"$ Steel Load Plate

¾" Thick Embedded Steel Sole Plate (See Sheet S11)

-Bearing Seat (Grind Level Under Bearing)



TYPICAL SECTION

• Points Where Elevations are given

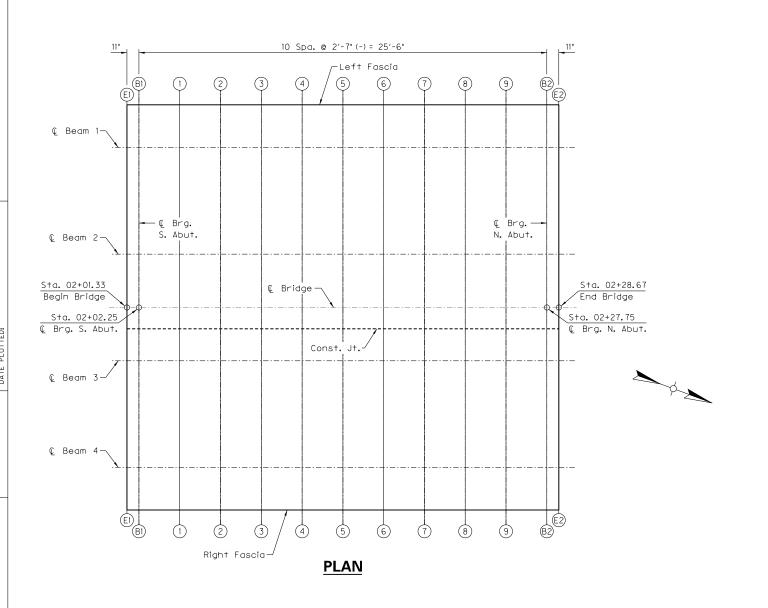


TABLE OF CONSTRUCTION ELEVATIONS													
LOCATION	E1	B1	1	2	3	4	5	6	7	8	9	B2	E2
LEFT FASCIA	914.439	914.440	914.446	914.452	914.457	914.462	914.467	914.470	914.474	914.476	914.479	914.481	914.483
CONSTR. EL	914.492	914.494	914.499	914.505	914.511	914.516	914.520	914.524	914.527	914.530	914.532	914.534	914.536
TOP BM. EL.													
DIMENSION X													
CONSTR. EL	914.625	914.627	914.632	914.638	914.643	914.648	914.652	914.656	914.659	914.662	914.665	914.668	914.669
TOP BM. 2 EL.													
DIMENSION X													
e poince	014 660	014 664	014 660	014 674	014 600	014 005	014 600	014 607	014 606	014 600	014 700	014 704	014 706
© BRIDGE	914.662	914.664	914.669	914.674	914.680	914.685	914.689	914.693	914.696	914.699	914.702	914.704	914.706
CONCT IT	914.655	914.657	914.662	914.668	914.673	914.678	914.682	914.686	914.689	914.692	914.695	914.698	914.699
CONST. JT.	314.633	314.631	314.002	314.666	314.613	314.010	314.002	314.000	314.003	314.632	314.633	314.636	314.633
CONSTR. EL	914.625	914.627	914.632	914.638	914.643	914.648	914,652	914,656	914.659	914.662	914.665	914.668	914.669
TOP BM. 3 EL.	314.023	317.021	317.032	314.030	JIT. 073	317.070	317.032	314.030	314.033	J17.002	314.003	317.000	317.003
DIMENSION X													
BIMERIOION X													
CONSTR. EL	914.492	914,494	914.499	914,505	914.511	914.516	914.520	914.524	914.527	914.530	914.532	914.534	914.536
TOP BM. 4 EL.													
DIMENSION X													
RIGHT FASCIA	914.439	914.440	914.446	914.452	914.457	914.462	914.467	914.470	914.474	914.476	914.479	914.481	914.483

NOTES FOR ELEVATIONS TAKEN ON BEAMS

Take elevations on top of beam at points indicated by the grid layout. The beam elevations are to be read to three decimals, and entered in tables under "Top of Beam" elevations.

Compute dimensions "X" as follows: "Construction Elevation" minus "Top of Beam" elevation equals dimension "X". Construction Elevations include camber due to weight of the concrete slab and railing. Measuring of dimension "X" gives the final check on beam tolerances for camber, beam damage, and errors in erection that produce reverse cambers, sags, and unsightly fascia beams. In no case shall dimension "X" values be less than 8" or greater than 12". If the elevations of the existing bridge are such that these criteria cannot be obtained, the profile grade shall be adjusted. Coordinate adjustment with the Engineer.

For setting templates, measure dimension "X" above top of beam for top of template. Do not set template by elevations.

Temporary supports or shoring will not be permitted under the girders when pouring the concrete floor slab or when taking "Top of Beam" elevations.

Construct railing to roadway grade. Do not add camber to the railing.

Phase 3 construction joint elevation shall match the constructed Phase 2 construction joint elevation. The adjustment to the construction joint shall also be applied to other lines in Phase 3 to maintain a constant cross slope.

Note to Resident: The "Maximum Allowable Camber" shown on the beam sheet is the amount of camber, measured prior to casting the deck, above which the beam will begin to encroach into the slab.

NOTES

1. See Sheet SO4 for Parabolic Crown Detail.

REVISION		DATE
DATE: 8/23/2019	CHECKED E	3 Y
DESIGNED BY: K. Compton	T. Baker	
DETAILED BY: A. Foley	K. Compton	

Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS

CLAY

US-421 BRANCH OF ISLAND CREEK

CONSTRUCTION ELEVATIONS

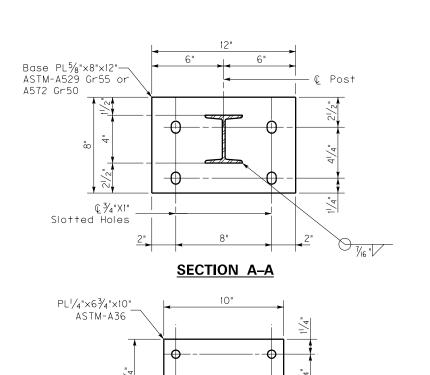
BRIDGE NUMBER

026B00002N





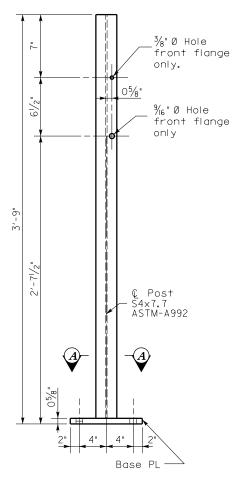
Lock Washer placed under two Hex Nuts. (Tighten the first hex nut by hand until the top and bot. edges of the W-beam engage the backer plate snug against the post. Then tighten hex nut one revolution with wrench and -secure with the 2nd hex nut.) Nominal Face of Rail — ASTM A-36 $PL^{1}/8$ "×1 $^{3}/4$ "×1 $^{3}/4$ " with $\frac{3}{8}$ " Ø hole centered in PL. -S4x7.7 Post W-Beam Backer PL $\mathbb{Q}^{1/2}$ " Ø $\times 1^{1/4}$ " Hex Head 05%" ASTM-A307 with one Hex Nut ASTM-A563 € Bridge € Post Ex. Asphalt— Overlay/ Ex. Asphalt Overlay Phase 2 Removal Line $\mathbb{Q}^{1/8}$ " Ø formed holes for 5/8" Ø Heavy Hex 4" | Head Anchor Bolts ASTM-A325 or A449 with one Hardened Washer Existing Bridge Deck and one Regular Lock -Washer PL Washer placed under X-SECTION VIEW each Heavy Hex Nut TRAFFIC VIEW ASTM-A563



WASHER PLATE

© "/16 " Ø

Holes



POST ELEVATION

NOTES

- These details are modified from Std. Dwg. BHS-012, Railing System Type T631 Details. For details not shown here, see Std. Dwg. BHS-012.
- 2. These modifed Railing System Type T631 details are to be used only where shown on Sheet SO5.

Campanica alth at Wantushi						
DETAILED BY: K. Compton	T. Baker					
DESIGNED BY: K. Compton	T. Baker					
DATE: 8/23/2019	CHECKED E	3 Y				
REVISION		DATE				

Commonwealth of Kentucky **DEPARTMENT OF HIGHWAYS**

CLAY

US-421 **BRANCH OF ISLAND CREEK**

MODIFIED RAILING SYSTEM TYPE T631

BRIDGE NUMBER 026B00002N



